# ANALYSIS OF MECHANICAL SYSTEMS USING INTERVAL COMPUTATIONS APPLIED TO FINITE ELEMENT METHODS 

O. Dessombz, F. Thouverez, J.-P. Laîné and L. Jézéquel<br>Laboratoire de Tribologie et Dynamique des Systèmes, Ecole Centrale de Lyon, B.P. 163, 69131 Ecully Cedex, France

(Received 21 September 1999, and in final form 24 March 2000)


#### Abstract

This paper addresses the problem of mechanical systems in which parameters are uncertain and bounded. Interval calculation is used to find an envelope of transfer functions for mechanical systems modelled with finite elements. Within this context, a new formulation has been developed for finite element problems involving bounded parameters, to avoid the problems of overestimation. An iterative algorithm is introduced, which leads to a conservative solution for linear mechanical problems. A method to ensure the convergence of this algorithm is also proposed. This new algorithm has been tested on simple mechanical systems, and leads to a conservative envelope of the transfer functions.


C 2001 Academic Press

## 1. INTRODUCTION

The physical parameters used to describe a structure are often uncertain, due to physical and geometrical uncertainties, or modelling inaccuracies. They are, for instance, Young's modulus, Poisson's ratio, length, volumic mass or thickness of plates. These uncertain parameters are generally identified by random variables, and introduced in a stochastic approach of the problems. Different methods can be used to solve these stochastic problems. A Monte Carlo simulation may, for example, be carried out. Several other methods exists [1], such as the perturbation method, the Neumann expansion series, or a projection on homogeneous chaos. But all these methods consider stochastic variables for which the density of probability is known (Gaussian variables are mostly used). Furthermore, real variables are bounded, which is not the case for most stochastic variables. The Monte Carlo method is very expensive from a CPU point of view, and the others often encounter convergence problems. Moreover, only the mean value and the moments (often the variance only) are known, and since the density of probability of the solution is not known, these informations are difficult to use. As most of the time the variables can be bounded, it seems to be judicious to investigate the mechanical problems containing uncertain parameters from the interval arithmetic theory point of view. Thus, interval arithmetic [2-4] will be applied in connection with the finite element methods.

We are interested in solving linear system of equations, which correspond to the classical mechanical problem of finding the transfer function of a structure,

$$
\begin{equation*}
\left([K](1+\mathrm{i} \eta)-\omega^{2}[M]\right)[H]=[I] \tag{1}
\end{equation*}
$$

where $[K]$ and $[M]$ are the stiffness and mass matrices respectively, $\eta$ the coefficient of hysteretic damping, $\omega$ the excitation frequency, $[H]$ is the dynamic compliance matrix, and [I] the identity matrix.

These problems have already been studied by several researchers (e.g., by Chen and Ward [5], Dimarogonas [6], and Koyluoglu [7]). They applied numerical methods developed for "reliable computing" based on interval matrices algebra [8-10]. Elishakoff et al. have focused on the bounds of eigenvalues of such dynamic systems [11-15]. Chen [5] has pointed out the limitations of these formulations, which present a major drawback: the classical formulation does not take into account the way the matrices are built for mechanical problems. In fact, the terms of the matrices are not independent of each other, since they are calculated from the same parameters, for instance, Young's modulus or density.

We will first introduce some basic concepts about interval arithmetic, and we will present the problematic of solving linear systems of interval equations. We will then introduce a new formulation of the problem, based on interval parameters which is adapted for the modelling of mechanical systems.

An adaptation of Rump's algorithm [9] will be proposed which takes into account this novel interval formulation. The new algorithm is iterative, and therefore the convergence criteria will be evaluated. The algorithm will be tested on a simple case to enable a comparison with the classical formulation. We will finally study frequency response functions for different mechanical systems, and also evaluate the amount of computations on simple discrete systems, as well as the accuracy of the solutions.

## 2. RESOLUTION OF INTERVAL LINEAR SYSTEMS

The interval arithmetic has been first introduced by Moore [2], who was interested in the error propagation due to truncation of the mantissa in computers. Many publications (in particular the book of Alefeld and Herzberger [3]) give the basic and advanced concepts of this theory.

In this paper, boldface, lower cases, underscores and overscores, respectively, denote intervals, scalars, lower and upper bounds of intervals:

$$
\begin{equation*}
\mathbf{x}=[\underline{\mathbf{x}}, \overline{\mathbf{x}}] . \tag{2}
\end{equation*}
$$

The basic interval operations are presented in Appendix A. The interval arithmetic has special properties (in particular, the property of sub-distributivity $(x)(\mathbf{y}+\mathbf{z}) \subseteq \mathbf{x y}+\mathbf{x z}$ ) that can lead to problems of overestimation when evaluating functions. We shall then be mindful to that problem in this paper.

One can also define interval vectors and interval matrices. Interval matrices can be expressed as

$$
\begin{equation*}
[\mathbf{A}]=\left[A_{c}\right]+[-[\operatorname{rad}([\mathbf{A}])],[\operatorname{rad}([\mathbf{A}])]], \tag{3}
\end{equation*}
$$

which is quite a convenient form.
The special properties of interval matrices have been investigated, for example, by Ning et al. and Rohn [10, 16]. (A list of nomenclature is given in Appendix D.)

## 2．1．SOLVING LINEAR SYSTEMS

If one is interested in the dynamic behavior of an industrial mechanical structure，one has to consider finite element modelling，which leads to matrices（such as stiffness，mass，or damping matrix）．Thus，finding frequency response functions corresponds to solving linear systems of equations．If some of the mechanical parameters are uncertain at design stage， they can be modelled by using the interval theory．The uncertain parameters can be geometrical ones（length，thickness，clearance，etc．），or physical ones（Young＇s modulus， etc．）．Then the matrices given by the finite element theory are interval matrices，and the problem is generally（static problems，frequency response functions）written as：

$$
\begin{equation*}
[A]\{x\}=\{b\} \tag{4}
\end{equation*}
$$

with $[A] \in[\mathbf{A}]$ and $\{b\} \in\{\mathbf{b}\}$ ．Although several problems can be distinguished，as done by Chen and Ward［5］and by Shary［17］，we will focus exclusively in this paper on the solution set of the outer problem which is defined as $\Sigma_{\exists \exists}([\mathbf{A}],\{\mathbf{b}\})$ ：

$$
\begin{equation*}
\Sigma_{\exists \exists}([\mathbf{A}],\{\mathbf{b}\})=\left\{x \in \mathbb{R}^{n} \mid(\exists[A] \in[\mathbf{A}]),(\exists\{b\} \in\{\mathbf{b}\}) /[A]\{x\}=\{b\}\right\}, \tag{5}
\end{equation*}
$$

where［A］is an interval matrix and $\{\mathbf{b}\}$ an interval vector．
In general，this set is not an interval vector．It is a non－convex polyhedra（see reference［5］ or reference［17］，for examples）．The Oettli and Prager theorem［18］gives an expression to get the exact solution set（5）．

Theorem 2.1 （Oettli and Prager Lemma）．Let $[\mathbf{K}] \in \mathbb{I} \mathbb{R}^{n \times n}$ and $\{\mathbf{f}\} \in \mathbb{I} \mathbb{R}^{n}$ ．Then

$$
\begin{equation*}
\{x\} \in \Sigma_{\exists \exists}([\mathbf{K}],\{\mathbf{f}\}) \Leftrightarrow|m([\mathbf{K}])\{x\}-m(\{\mathbf{f}\})| \leqslant \operatorname{rad}([\mathbf{K}])|\{x\}|+\operatorname{rad}(\{\mathbf{f}\}) . \tag{6}
\end{equation*}
$$

Nevertheless，this expression is quite difficult to use with matrices corresponding to real physical cases in an $n$－dimensional problem．Most of the time，only the smallest interval vector containing $\Sigma_{\exists \exists}([\mathbf{A}],\{\mathbf{b}\})$ will be considered，which is defined as $\square \Sigma_{\exists \exists}([\mathbf{A}],\{\mathbf{b}\})$ ．In this case，this ensures that the true solution is included in the numerical solution found $\square \Sigma_{\exists \exists}([\mathbf{A}],\{\mathbf{b}\})$ ．Within the context of this problematic，equation（4）can be rewritten as

$$
\begin{equation*}
[\mathbf{A}] x=\{\mathbf{b}\} . \tag{7}
\end{equation*}
$$

Several algorithms intend to solve this problem．For example，the Gaussian elimination algorithm can be adapted to the resolution of a linear system whose coefficients are interval． Alefeld has given some basic results in reference［3］．Rohn has shown in reference［19］that this algorithm could lead to an important overestimation of the solution．It even sometimes cannot solve the system because of zero pivot encountered．

Ning and Kearfott have made a review in reference［10］of existing methods for finding either $\square \Sigma_{\text {ヨヨ }}([\mathbf{A}],\{\mathbf{b}\})$ or an interval vector containing $\square \Sigma_{\text {ヨヨ }}([\mathbf{A}],\{\mathbf{b}\})$ ．These methods use particular forms of the matrices，that do not exactly correspond to mechanical cases，and are more appropriate for the treatment of numerical uncertainties as they are not well suited for dealing with large uncertainties．

Another useful method is based on a residual iteration，it is called the inclusion method of Rump［9］．It is an iterative method relying on the fixed point theorem，that leads to sharp results quite fast．

## 3. FORMULATION ADAPTED TO FINITE ELEMENT METHODS

The existing algorithms used to solve $\Sigma_{\exists \exists}([\mathbf{A}],\{\mathbf{b}\})$ have been formulated for reliable computing from a numerical point of view. In an interval matrix for instance, each term can vary independently of each other in its interval, which is generally sharp.

If the interval formulation has to be adapted to mechanics, the dependence between the parameters must be taken into account, because many of the terms of the matrices are depending on the same parameters. For example, if the Young's modulus varies in E, a stiffness matrix could formally be written as

$$
\mathbf{E}\left[\begin{array}{ll}
k_{11} & k_{12}  \tag{8}\\
k_{21} & k_{22}
\end{array}\right]
$$

which is not the same as

$$
\left[\begin{array}{ll}
\mathbf{E} k_{11} & \mathbf{E} k_{12}  \tag{9}\\
\mathbf{E} k_{21} & \mathbf{E} k_{22}
\end{array}\right]
$$

that is treated in the classical interval techniques as

$$
\left[\begin{array}{ll}
E 1 k_{11} & E 2 k_{12}  \tag{10}\\
E 3 k_{21} & E 4 k_{22}
\end{array}\right]
$$

with $E 1, E 2, E 3, E 4$ varying in $\mathbf{E}$ independently.
When including the parameters in the terms of the matrices and vectors, the width of $\Sigma_{\exists \exists}([\mathbf{A}],\{\mathbf{b}\})$ grows substantially (see the example in section 4.2). If all the matrices $[K] \in[\mathbf{K}]$ are considered, it must be noticed that many of them do not physically correspond to the stiffness matrices, because stiffness matrices are symmetric positive and definite. For the different interval parameters in the matrix [A] to be put into factor as in equation (8), $[\mathbf{A}]$ and $\{\mathbf{b}\}$ are developed as follows:

$$
\begin{equation*}
[\mathbf{A}]=\left[A_{0}\right]+\sum_{n=1}^{N} \boldsymbol{\varepsilon}_{\mathbf{n}}\left[A_{n}\right], \quad\{\mathbf{b}\}=\left\{b_{0}\right\}+\sum_{p=1}^{P} \boldsymbol{\beta}_{\mathbf{p}}\left\{b_{p}\right\} . \tag{11}
\end{equation*}
$$

$N$ and $P$ are the number of interval parameters to be taken into account when building the matrix [A] and the vector $\{\mathbf{b}\} . \boldsymbol{\varepsilon}_{\mathbf{n}}$ and $\boldsymbol{\beta}_{\mathbf{p}}$ are independent centered intervals, generally $[-1,1]$. $\left[A_{0}\right]$ and $\left\{b_{0}\right\}$ correspond to the matrices and vector built from the mean values of the parameters.

For a mechanical problem, the stiffness matrix will be written with factorized parameters

$$
\begin{equation*}
[\mathbf{K}]=\left[K_{0}\right]+\sum_{n=1}^{N} \varepsilon_{\mathrm{n}}\left[K_{n}\right] . \tag{12}
\end{equation*}
$$

For each value of $\varepsilon_{n}$ in $\varepsilon_{n}$, [K] remains symmetric positive and definite, due to the physical character of the parameters.

## 4. A NEW ALGORITHM FOR THE SOLUTION

For the particular form of the problem shown in equation (11), where the interval parameters are put into factor in front of the matrices, it is necessary to adapt the algorithms. The new algorithm of resolution proposed here relies on Rump's technique, that
has been presented by Rohn in reference [20]. His demonstration is recalled in Appendix B. The inclusion method of Rump [9] relies on the fixed point theorem, and had to be adapted to avoid the problems of overestimation due to the loss of dependence in interval arithmetic. As the basic method of Rump, our algorithm is iterative, and then subject to convergence criteria that will be analyzed in section 4.1.

First, consider a system in which only one parameter is an interval; then

$$
\begin{equation*}
[\mathbf{A}]=\left[A_{0}\right]+\alpha\left[A_{1}\right], \quad \alpha \text { is centered } \tag{13}
\end{equation*}
$$

is the equation of the system.
The implementation of the algorithm is as follows.
First, an initialization stage:
$\boldsymbol{\varepsilon}=[0 \cdot 9,1 \cdot 1]$ is the so-called inflation parameter;
$[R]=\operatorname{inv}(\operatorname{mid}[\mathbf{A}])=\left[A_{0}\right]^{-1}$ is an estimation of the inverse of $\operatorname{mid}[\mathbf{A}] ;$
$\left\{x_{s}\right\}=[R] *\{b\}$ is an estimation of the solution;
$[B]=\left[A_{0}\right]^{-1}\left[A_{1}\right]$;
$\{\mathbf{g}\}=[R] *\left(\{b\}-[\mathbf{A}] *\left\{x_{s}\right\}\right)=-\boldsymbol{\alpha}\left[A_{0}\right]^{-1}\left[A_{1}\right]\left[A_{0}\right]^{-1}\{b\}=-\boldsymbol{\alpha}[B]\left\{x_{s}\right\} ;$
$\left\{\mathbf{x}_{0}\right\}=\{\mathbf{g}\}$ initialization of the solution $\left\{\mathbf{x}^{*}\right\}$;
$[\mathbf{G}]=[I]-[R] *[\mathbf{A}]=-\alpha[B]$ is the iteration matrix in the equation

$$
\begin{equation*}
\left\{\mathbf{x}^{*}\right\}=[\mathbf{G}]\left\{\mathbf{x}^{*}\right\}+\{\mathbf{g}\} . \tag{14}
\end{equation*}
$$

Second, iterative resolution:
$\{\mathbf{y}\}=\boldsymbol{\varepsilon} *\{\mathbf{x}\} ;$
$\{\mathbf{x}\}=\{\mathbf{g}\}+[\mathbf{G}] *\{\mathbf{y}\}$
until $\{\mathbf{x}\} \subset\left\{\mathbf{y}^{0}\right\}$;
if the condition $\{\mathbf{x}\} \subset\left\{\mathbf{y}^{0}\right\}$ is satisfied, then $\{\mathbf{x}\}$ is a conservative solution of the equation $[\mathbf{A}]\{\mathbf{x}\}=\{\mathbf{b}\}$.

It must be noticed that all the matrices multiplications and linear system resolutions concern only deterministic matrices (opposed to the interval ones). The interval formulation is preserved, and the interval parameters are put into factor in front of deterministic matrices. The control of the intervals is essential to avoid large overestimations of the solutions.

After $n$ iterations, the solutions are given by the equations

$$
\begin{gather*}
\left\{\mathbf{y}_{n}\right\}=\left\{\mathbf{y}_{n-1}\right\}+(-1)^{n} \boldsymbol{\alpha}^{n} \varepsilon^{n}[B]^{n}\left\{x_{s}\right\},  \tag{15}\\
\left\{\mathbf{x}_{n}\right\}=\left\{\mathbf{x}_{n-1}\right\}+(-1)^{n+1} \boldsymbol{\alpha}^{n+1} \boldsymbol{\varepsilon}^{n}[B]^{n+1}\left\{x_{s}\right\}, \tag{16}
\end{gather*}
$$

where the interval parameters have been put in factor in front of the deterministic matrices.
The main difference from the algorithm of Rump is the control of the interval parameters inside the iterative scheme, that avoids dramatic overestimations.

Rohn and Rex have shown in references [21,20] that the algorithm converges if and only if $\rho(|[\mathbf{G}]|)<1$, where $\rho(|[G]|)$ is the spectral radius of the absolute value of [G]. Few iterations are necessary to get a result if the matrix [G] is contracting. If the number of iterations remains small, the overestimation of the solution is not important, and that is why making [G] as much contracting as possible is interesting: it reduces the number of iterations and by the way the overestimation effect.

The method proposed above on a system with one interval parameter can easily be generalized to the problems where

$$
[\mathbf{A}]=\left[A_{0}\right]+\sum_{i=1}^{N 1} \boldsymbol{\alpha}_{i}\left[A_{i}\right], \quad\{\mathbf{b}\}=\left\{b_{0}\right\}+\sum_{j=1}^{N 2} \boldsymbol{\beta}_{j}\left\{b_{j}\right\} .
$$

### 4.1. CONVERGENCE OF THE METHOD

We have proposed an iterative algorithm for solving the linear systems with interval parameters. This algorithm is based on the fixed point theorem, and the iteration matrix must be contracting. The problem of the convergence of the algorithm is then crucial to get solutions.

We have seen that the equation

$$
\begin{equation*}
\left\{\mathbf{x}^{*}\right\}=[\mathbf{G}]\left\{\mathbf{x}^{*}\right\}+\{\mathbf{g}\} \tag{17}
\end{equation*}
$$

is convergent if and only if $\rho(|[\mathbf{G}]|)<1$. In the general case, the iteration matrix is given by

$$
\begin{equation*}
[\mathbf{G}]=\sum_{i=1}^{N 1} \mathbf{e}_{i}\left[A_{0}\right]^{-1}\left[A_{i}\right] \tag{18}
\end{equation*}
$$

and the condition is

$$
\begin{equation*}
\rho\left(\left|\sum_{i=1}^{N 1}-\mathbf{e}_{i}\left[A_{0}\right]^{-1}\left[A_{i}\right]\right|\right)<1 \tag{19}
\end{equation*}
$$

which is quite difficult to evaluate.
To estimate this value, one can use the following Theorem 4.1 (see reference [22]).
Theorem 4.1 (Perron-Froebenius). Let $[A]$ and $[B]$ be two $n \times n$ matrices with $0 \leqslant|[B]| \leqslant[A]$. Then,

$$
\begin{equation*}
\rho([B]) \leqslant \rho([A]) . \tag{20}
\end{equation*}
$$

And as it is well known that $|A+B| \leqslant|A|+|B|$, one can say that if the stronger convergence condition

$$
\begin{equation*}
\rho\left(\sum_{i}\left|-\mathbf{e}_{i}\right|\left|\left[A_{0}\right]^{-1}\left[A_{i}\right]\right|\right)<1 \tag{21}
\end{equation*}
$$

is verified, then equation (19) is also true.
The condition $\rho(|[\mathbf{G}]|)<1$ is not always true, especially for systems with wide interval parameters. We propose a method to avoid this problem and also to improve the contracting level of [G].

For a system with one interval parameter $\left(\left[A_{0}\right]+\mathbf{e}\left[A_{1}\right]\right)\{\mathbf{x}\}=\{b\}$, the iteration matrix is

$$
\begin{equation*}
[\mathbf{G}]=-\mathbf{e}\left[A_{0}\right]^{-1}\left[A_{1}\right] \tag{22}
\end{equation*}
$$

and the condition of convergence is

$$
\begin{equation*}
\rho(|[\mathbf{G}]|)=|\mathbf{e}| \rho\left(\left|\left[A_{0}\right]^{-1}\left[A_{1}\right]\right|\right)<1 . \tag{23}
\end{equation*}
$$

$\mathbf{e}$ is a centered interval, so that $\left[A_{0}\right]$ is the mean value of $\left[A_{0}\right]+\mathbf{e}\left[A_{1}\right]$.
If $\mathbf{e}$ is a relatively wide interval (it means that the terms of $\mathbf{e}\left[A_{1}\right]$ are relatively wide with respect to the corresponding terms in $\left[A_{0}\right]$ ), condition (23) can be false and the algorithm will be divergent. If this is the case, the strategy proposed is to split the interval into a partition of it, and then work on narrower intervals, on which condition (23) will be verified. If one considers a partition of the interval $\mathbf{e}=\cup \mathbf{e}_{i}$, one has

$$
\begin{equation*}
\Sigma_{\exists \exists}\left(\left[A_{0}\right]+\mathbf{e}\left[A_{1}\right],\{b\}\right)=\bigcup_{i} \Sigma_{\exists \exists}\left(\left[A_{0}\right]+\mathbf{e}_{i}\left[A_{1}\right],\{b\}\right) . \tag{24}
\end{equation*}
$$

From $\mathbf{e}$ to $\mathbf{e}_{i},\left[A_{0}\right]$ becomes $\left[A_{0}\right]+m\left(\mathbf{e}_{i}\right)\left[A_{1}\right]$, and $\left[A_{1}\right]$ remains the same. The equation to be solved is

$$
\begin{equation*}
\left(\left[A_{0}\right]+m\left(\mathbf{e}_{i}\right)\left[A_{1}\right]+\left[-\operatorname{rad}\left(\mathbf{e}_{i}\right), \operatorname{rad}\left(\mathbf{e}_{i}\right)\right]\left[A_{1}\right]\right)\left\{\mathbf{x}_{i}\right\}=\{b\} . \tag{25}
\end{equation*}
$$

One can define $d$ as

$$
\begin{equation*}
d=\sup _{\mathbf{e}_{i} \subset \mathbf{e}}\left(\rho\left(\left|\left(\left[A_{0}\right]+m\left(\mathbf{e}_{i}\right)\left[A_{1}\right]\right)^{-1}\left[A_{1}\right]\right|\right)\right) . \tag{26}
\end{equation*}
$$

For all interval $\mathbf{e}_{i}$ such that $w\left(\mathbf{e}_{i}\right)<1 / d$,

$$
\begin{equation*}
\left|\mathbf{e}_{i}\right| \rho\left(\left|\left[A_{0}\right]^{-1}\left[A_{1}\right]\right|\right)<1 \tag{27}
\end{equation*}
$$

It is then possible to split the interval $\mathbf{e}$ into a partition of it $\bigcup_{i} \mathbf{e}_{i}$, where the algorithm is convergent for each interval $\mathbf{e}_{i}$.

For multi-interval parameter problems, the same kind of splitting technique can be used, leading to the same result. Moreover, this technique can also be used to accelerate the convergence of the iterative scheme. The smaller the spectral radius of $|[G]|$, the faster the convergence of the algorithm and the smaller the overestimation of the solution.

### 4.2. TEST OF THE NEW ALGORITHM ON A SIMPLE CASE

A new version of the algorithm of Rump has been developed to handle the case in which the interval parameters are put into factor in front of the matrices. The intervals are then controlled all along the algorithm, to avoid too large an overestimation. Moreover, the convergence of the algorithm can be guaranteed, and even improved by splitting the intervals into a partition of them.

One can now test the proposed algorithm on a simple case to emphasize its efficiency with respect to the basic method.

A new interval formulation adapted to the mechanical problems has been proposed. The results found with the modified Rump's algorithm are often much sharper than the ones found with the classical formulation. To show the efficiency of the method for finding the solution of a linear system $[\mathbf{A}]\{x\}=\{\mathbf{b}\}$, one can consider the very simple example of a clamped free beam (see Figure 1).
$F$ and $M$ are, respectively, the shear force and bending momentum applied at the free end of the beam, $d$ and $\theta$ correspond to the displacement and slope at the free end of the beam.


Figure 1. Clamped free beam.

The characteristics of the beam are as follows:

$$
\begin{gather*}
\text { Young's modulus } E \in[2 \cdot 058 e 11,2 \cdot 142 e 11] \quad(2 \cdot 1 e 11 \pm 2 \%),  \tag{28}\\
\text { inertia } I \in[8 \cdot 82 e-8,9 \cdot 18 e-8] \quad(9 e-8 \pm 2 \%),  \tag{29}\\
\text { length } l=1 \tag{30}
\end{gather*}
$$

The shear force and bending momentum are also interval parameters:

$$
\{\mathbf{f}\}=\left\{\begin{array}{c}
{[-10 \cdot 2,-9 \cdot 8]}  \tag{31}\\
{[29 \cdot 4,30 \cdot 6]}
\end{array}\right\} .
$$

If one considers the elementary finite element matrix of the Euler-Bernouilli theory [23], the static matrix equation of the problem is

$$
\left[\begin{array}{cc}
\frac{2 E I}{9 l} & \frac{-E I}{3 l^{2}}  \tag{32}\\
\frac{-E E I}{3 l^{2}} & \frac{2 E I}{3 l^{2}}
\end{array}\right]\left\{\begin{array}{l}
d \\
\theta
\end{array}\right\}=\left\{\begin{array}{l}
F \\
M
\end{array}\right\},
$$

and from a numerical point of view, the stiffness matrix is an interval matrix:

$$
\left[\begin{array}{cc}
{[4033 \cdot 68,4369 \cdot 68]} & {[-6554 \cdot 52,-6050 \cdot 52]}  \tag{33}\\
{[-6554 \cdot 52,-6050 \cdot 52]} & {[12101 \cdot 4,13109 \cdot 04]}
\end{array}\right]
$$

The first problem that can be solved is finding the solution set corresponding to the numerical equation

$$
\left[\begin{array}{cc}
{[4033 \cdot 68,4369 \cdot 68]} & {[-6554 \cdot 52,-6050 \cdot 52]}  \tag{34}\\
{[-6554 \cdot 52,-6050 \cdot 52]} & {[12101 \cdot 04,13109 \cdot 04]}
\end{array}\right]\left\{\begin{array}{l}
d \\
\theta
\end{array}\right\}=\left\{\begin{array}{c}
{[-10 \cdot 2,-9 \cdot 8]} \\
{[29 \cdot 4,30 \cdot 6]}
\end{array}\right\} .
$$

The Oettli and Prager lemma gives the exact solution set $\Sigma_{\exists \exists}([\mathbf{A}],\{\mathbf{b}\})$ and $\square \Sigma_{\exists \exists}([\mathbf{A}],\{\mathbf{b}\})$ (dotted line) which is shown in Figure 2. All the terms in the matrix are said to be independent. Consider the mechanical problem with factorized interval parameters:

$$
\mathbf{E I}\left[\begin{array}{cc}
2 / 9 l & -1 / 3 l^{2}  \tag{35}\\
-1 / 3 l^{2} & 2 / 3 l^{3}
\end{array}\right]\left\{\begin{array}{l}
d \\
\theta
\end{array}\right\}=\left\{\begin{array}{l}
\mathbf{F} \\
\mathbf{M}
\end{array}\right\} .
$$

As this system is quite simple, the solution can be found analytically. The exact mechanical solution set is given in Figure 2. It is called the mechanical exact solution set. The hull of this


Figure 2. Solution sets for the clamped free beam. $E I$ is uncertain ( $\pm 2 \%$ ). Numerical global problem, and reduced mechanical problem, and their respective hulls.
set (which is an interval vector) has also been drawn. The mechanical exact solution set is included in $\Sigma_{\exists \exists}([\mathbf{A}],\{\mathbf{b}\})$, and is really small in comparison. This shows how important the factorization is for solving mechanical problems.

To test the algorithm, the result of the modified Rump's algorithm has been computed. It is illustrated in Figure 2. As one can see, it is overestimating the exact solution, but it gives a good idea of the size of the solution. Above all, it is really smaller than the range computed when considering all the terms in the matrices independent, as in the initial Rump's algorithm.

As it has been noticed in reference [5], a large overestimation is obtained when including the parameters in the elements of the matrices. For finite element matrices, this overestimation can become critical, and often leads to an insolvable problem. As it has been shown above, even on $2 \times 2$ matrices, the overestimation can reach 10 times or more. Such an adaptation of this algorithm enables its use for industrial problems involving huge size matrices.

## 5. APPLICATIONS ON MECHANICAL SYSTEMS

Several specific examples are now presented to show the efficiency of the new algorithm. Each one is associated with a particular difficulty, for instance the number of parameters, or the development of the matrices into a sum with interval parameters put into factor.

### 5.1. PROBLEM WITH SEVERAL PARAMETERS

This problem has two degrees of freedom, and is presented in Figure 3. The three stiffnesses are uncertain and vary in bounded intervals. The focus here is on finding the transfer function envelope of the system.


Figure 3. Three Springs system. $x_{1}$ and $x_{2}$ are the displacements of the masses $m_{1}$ and $m_{2}$, that are subject to the forces $f_{1}$ and $f_{2}$ respectively.

Table 1
Numerical values of the parameters

| $k_{1}^{0}=100 \mathrm{Nm}^{-1}$ | $k_{2}^{0}=10 \mathrm{Nm}^{-1}$ | $k_{3}^{0}=100 \mathrm{Nm}^{-1}$ |
| :---: | :---: | :---: |
| $k_{1}^{1}=0.04 k_{1}^{0}$ | $k_{2}^{1}=0.04 k_{2}^{0}$ | $k_{3}^{1}=0.04 k_{3}^{0}$ |
| $\eta_{1}=0.02$ | $\eta_{2}=0.02$ | $\eta_{3}=0.02$ |
| $m_{1}=1 \mathrm{~kg}$ | $m_{2}=1 \mathrm{~kg}$ |  |

Each spring of stiffness $k_{i}$ is subject to hysteretic damping $\eta_{i}$. Each value of the stiffness is uncertain ( $k_{i}=k_{i}^{0}+[-1,1] k_{i}^{1}$, or $k_{i}^{0}-k_{i}^{1}<k_{i}<k_{i}^{0}+k_{i}^{1}$ ).
The numerical values of the parameters are given in Table 1.
The dynamic problem is considered, and the set of equations for the transfer function is

$$
\begin{align*}
& \left(\left[\begin{array}{cc}
\left(1+\mathrm{i} \eta_{1}\right) k_{1}^{0}+\left(1+\mathrm{i} \eta_{2}\right) k_{2}^{0} & -\left(1+\mathrm{i} \eta_{2}\right) k_{2}^{0} \\
-\left(1+\mathrm{i} \eta_{2}\right) k_{2}^{0} & \left(1+\mathrm{i} \eta_{2}\right) k_{2}^{0}+\left(1+\mathrm{i} \eta_{3}\right) k_{3}^{0}
\end{array}\right]+\mathbf{e}_{1}\left[\begin{array}{cc}
\left(1+\mathrm{i} \eta_{1}\right) k_{1}^{1} & 0 \\
0 & 0
\end{array}\right]\right. \\
& \quad+\mathbf{e}_{2}\left[\begin{array}{cc}
\left(1+\mathrm{i} \eta_{2}\right) k_{2}^{1} & -\left(1+\mathrm{i} \eta_{2}\right) k_{2}^{1} \\
-\left(1+\mathrm{i} \eta_{2}\right) k_{2}^{1} & \left(1+\mathrm{i} \eta_{2}\right) k_{2}^{1}
\end{array}\right]+\mathbf{e}_{3}\left[\begin{array}{cc}
0 & 0 \\
0 & \left(1+\mathrm{i} \eta_{3}\right) k_{3}^{1}
\end{array}\right] \\
& \left.-\omega^{2}\left[\begin{array}{cc}
m_{1} & 0 \\
0 & m_{2}
\end{array}\right]\right)\left\{\begin{array}{l}
\mathbf{H}_{1} \\
\mathbf{H}_{2}
\end{array}\right\}=\left\{\begin{array}{l}
f_{1} \\
f_{2}
\end{array}\right\} . \tag{36}
\end{align*}
$$

The solution of this problem will be done on a frequency band including all the modes, represented by 61 points linearly spaced. The use of the new algorithm leads to envelope bounds of both real and imaginary parts of the transfer function for each frequency evaluated (see Figure 4). To have a very contracting iterative scheme, the spectral radius is imposed to be less than $0 \cdot 3$, and the inflation parameter is [0,2]. To compare the results with one of the Monte Carlo simulation, 10000 stochastic tests have been made on each of the 61 frequency points. The Monte Carlo simulation leads to an estimation of the envelope interval which is not conservative. One can then compare the results of both methods for a particular value of the frequency. For $\omega=9.5 \mathrm{rad} / \mathrm{s}$, the results are given in Table 2. The Monte Carlo simulation gives results that are included in the true bounds, whereas the proposed algorithm can find envelope bounds.

It must be noticed that the amount of computations for the proposed algorithm is small compared to the amount needed by the Monte Carlo simulation. If the example is


Figure 4. Real and imaginary parts of the collocated transfer function $(1,1)$ of the system shown in Figure 3. Solid lines represent the transfer function for several values of the stiffnesses. Crosses represent the envelope calculated with the modified Rump's algorithm, for $\pm 4 \%$ uncertainties.

Table 2
Real and imaginary parts of the collocated transfer function $(1,1)$ of the system shown in Figure 3, for $\omega=9 \cdot 5 \mathrm{rad} / \mathrm{s}$; the results of the proposed algorithm are compared with the results of the Monte Carlo simulations

|  | Monte Carlo <br> 5000 tests | Monte Carlo <br> 20000 tests | Proposed algorithm |
| :--- | :---: | :---: | :---: |
| Flops | 1380000 | 5520241 | 123500 |
| real $(H(1,1))$ | $[0.05020,0.09722]$ | $[0.05007,0.09755]$ | $[0.04829,0.09964]$ |
| imag $(H(1,1))$ | $[-0.02754,-0.00649]$ | $[-0.02773,-0.00644]$ | $[-0.02916,-0.00557]$ |

computed with a smaller uncertainty ( $\pm 2 \%$ for instance), the algorithm will be even faster (10 500 flops for $\omega=9.5 \mathrm{rad} / \mathrm{s}$ ). For the computation on all the 61 points, a $\pm 2 \%$ uncertainty on each spring will use 16.6 Mflops, and a simulation with $\pm 4 \%$ uncertainty 126.5 Mflops. The result is also quite good, the envelope is wrapping the deterministic transfer functions, without overestimating the true envelope too much (see Figures 4 and 5).

### 5.1.1. Non-linear dependence of the parameters

The decomposition of the finite element problems into a factorized sum as in equation (11) is not obvious. Consider a very simple finite element problem, that of a meshed clamped


Figure 5. Real and imaginary parts of the collocated transfer function $(1,1)$ of the system shown in Figure 3. Solid lines represent the transfer function for several values of the stiffnesses. Crosses represent the envelope calculated with the modified Rump's algorithm, for $\pm 2 \%$ uncertainties.


Figure 6. Clamped free plate meshed with 15 elements.
free plate whose thickness varies in an interval (see Figure 6). If $t$ is the thickness of the plates, both $t$ and $t^{3}$ appear in the elementary matrices of the Love-Kirchhoff theory (the stiffness matrix depends on $t^{3}$, and the mass matrix on $t$ ). The dynamic problem is written as

$$
\begin{equation*}
\left(\mathbf{t}^{3}[K](1+\mathrm{i} \eta)-\omega^{2} \mathbf{t}[M]\right)\{H\}=\{F\} . \tag{37}
\end{equation*}
$$



Figure 7. Collocated transfer function (real and imaginary parts) for the plate at node ( 1,1 ). Several deterministic transfer functions have been drawn, corresponding to different values of the thickness. The crosses correspond to the robust interval algorithm.

The intervals $\mathbf{t}$ and $\mathbf{t}^{3}$ cannot be considered as independent. Thus, one has to use an approximate expression to take this dependence into account. $\mathbf{t}$ can be written as $m(\mathbf{t})+[-1,1] \operatorname{rad}(\mathbf{t})$, or $t_{0}+\delta t$. Then $\mathbf{t}^{3}$ is

$$
\begin{equation*}
\left(t_{0}+\delta t\right)^{3}=t_{0}^{3}+3 \delta t \cdot t_{0}^{2}+3 t_{0} \cdot \delta t^{2}+\delta t^{3} \tag{38}
\end{equation*}
$$

The matrix equation (37) becomes

$$
\begin{align*}
& \left(\left(t_{0}^{3}[K](1+\mathrm{i} \eta)-\omega^{2} t_{0}[M]\right)+\delta t\left(3 \cdot t_{0}^{2}[K](1+\mathrm{i} \eta)-\omega^{2}[M]\right)\right. \\
& \left.\quad+\left(3 t_{0} \delta t^{2}+\delta t^{3}\right)[K](1+\mathrm{i} \eta)\right)\{H\}=\{F\}, \tag{39}
\end{align*}
$$

where $\delta t$ varies in $[-\operatorname{rad}(\mathbf{t}), \operatorname{rad}(\mathbf{t})]=[-\mathrm{d} t, \mathrm{~d} t]$. If $\delta t$ and $3 t_{0} \delta t^{2}+\delta t^{3}$ are said to be independent (which is false, but for $\delta t \ll t_{0}, \delta t \gg 3 t_{0} \delta t^{2}+\delta t^{3}$ ), one will get a new equation of the form.

$$
\begin{equation*}
\left(A_{0}+\varepsilon_{1} A_{1}+\varepsilon_{2} A_{2}\right) X=b, \tag{40}
\end{equation*}
$$

where $\boldsymbol{\varepsilon}_{1}=[-\mathrm{d} t, \mathrm{~d} t]$ and $\boldsymbol{\varepsilon}_{2}=\left[0,3 t_{0} \mathrm{~d} t^{2}+\mathrm{d} t^{3}\right]$. Equation (39) has been modified so that the dependence between the preponderant terms is conserved (i.e., the terms in $\delta t$ ). The other terms are then considered to be independent of $\delta t$. Taking these terms into account is


Figure 8. Zoom of the collocated transfer function for the plate.
anyway essential for the algorithm to lead to conservative results. By treating the new equation with the modified Rump's algorithm, one can get a conservative result of the transfer function of the plate.

The numerical example treated is a clamped free plate (dimensions $4 \mathrm{~m} \times 1 \mathrm{~m}$ ), whose thickness is $t=5 \times 10^{-2} \mathrm{~m} \pm 6 \%$. The value of the hysteretic damping in the plate is $2 \%$. The plate is meshed with $5 * 3$ elements (see Figure 6).

The collocated transfer function calculated in point (1, 1) (see Figure 6) which is represented in Figures 7 and 8. The algorithm leads to an envelope of the real and imaginary parts of the transfer function, and the overestimation remains small.

### 5.1.2. System with multiple eigenvalues

A last example will be treated to show the efficiency of the interval calculus, when taking into account small uncertainties that are inherent in mechanical systems. The new algorithm permits in this case to bring out important effects due to these small fluctuations.

Consider a three bladed-disk that is modelled with a seven-degrees-of-freedom (7-d.o.f.) system (see Figure 9). The blades are modelled with the Euler-Bernouilli theory, and only hysteretic damping is considered. The values of the parameters are: length $L=1 \mathrm{~m}$, area $S=\pi 10^{-4} \mathrm{~m}^{2}$, Young's modulus $E_{0}=210 \mathrm{GPa}$, and volumic mass $\rho=7800 \mathrm{~kg} / \mathrm{m}^{3}$. The damping coefficient is $\eta=2 \%$. Young's modulus of one blade is uncertain ( $E=E_{0} \pm 10 \%$ ). As the three blades are identical in the crisp tuned model, the eigenfrequencies are found as


Figure 9. Three bladed-disk, and the 7 d.o.f.


Figure 10. Modulus of the transfer function $H(1,3)$. Young's modulus of the first blade is uncertain ( $E=E_{0} \pm 10 \%$ ). Dashed line represent the deterministic case for which all the blades are identical, solid lines are the envelope calculated with the proposed algorithm.
multiple eigenvalues of a matrix system. If one of the blades is mistuned, then the eigenvalues are no longer multiple ones, and new resonances can appear. This is a complete modification of the structure, and that kind of phenomenon is well known in aeronautics (see references $[24,25]$ ) and can lead to the appearance of a much stronger dynamics than
the one expected for a tuned system. Consider the transfer function $H(1,3)$. When all the three blades are identical, the transfer function shows only two resonances. If one of the blades is mistuned (for instance, if its Young's modulus is not exactly the same as for the other blades) two new resonances appear on the transfer function.

In Figure 10 the modulus of the transfer function $H(1,3)$ is shown (its special calculation is explained in Appendix C). The dashed line represents the deterministic case for which all the three blades are identical, and solid lines the envelope of the transfer function for the system in which one blade has an uncertain Young's modulus ( $E=E_{0} \pm 10 \%$ ). The envelope shows four resonance zones. This is due to the mistuning phenomenon. This deep modification of the spectrum due to a small perturbation brings out the efficiency of the method, that can predict an a priori non-expected phenomena.

For this kind of computation too, the modified algorithm gives accurate results, once again with the advantage of getting a robust envelope. This method can improve considerably the accuracy of prediction of the dynamic behavior of mechanical systems involving inaccurate parameters.

## 6. CONCLUSION

Vibrating systems are often modelled by using a finite element method. When they are dependent on uncertain and bounded parameters, they can be studied owing to the interval calculus. For the solution of linear systems, in which some variables are intervals, one can find well-suited algorithms, but they consider only full interval matrices, whereas this does not correspond to real physical problems. A new formulation has been introduced here in which the interval parameters are factorized when building finite element matrices. For use with this factorized formulation, a novel algorithm is presented. It corresponds to a reformulation of the iterative algorithm of Rump [9] adapted to the finite element formulation. The convergence of this method has been studied, and a dichotomy scheme ensures the convergence of the algorithm. It is easy to notice on a simple example that the factorization and the proposed algorithm lead to better results than classical methods. On standard-sized finite element models, the classical methods would not work; hence the novel method proposed is interesting. This method enables one to find bounds of the transfer function of dynamic problems in which some of the parameters are uncertain and bounded. The relevance of such an envelope is that one can be certain that all the solutions corresponding to the bounded parameters are in this envelope. This is the robust aspect of the method.

If used in a design stage, this algorithm allows one to take into account from the beginning the uncertainties in the physical parameters of a product. Furthermore, if the algorithm is used for analysis, it will be possible, as the bounds of the physical parameters are known, to find guaranteed bounds for the static and dynamic responses. Then safety zones can be defined, where a given level of the responses will never be reached.

The algorithm is based on finite elements modelling, and the result is dependent on the accuracy of the numerical model. Moreover, as for classical deterministic FEM, the refinement of the mesh has some kind of influence on the solution. It is also necessary to take into account the model's errors in addition to the uncertainties in the parameters, but this is beyond the scope of this paper. The proposed algorithm can handle only a limited number of interval parameters. For working on industrial models, with tens or hundreds of uncertain parameters, the algorithm will have to be improved. But for a design stage, when few parameters are subjected to important uncertainties, it should really be useful.

## ACKNOWLEDGMENTS

The authors gratefully acknowledge the French Education Ministry for its support through grant No. 97089 for the investigation presented here.

## REFERENCES

1. R. G. Gnanem and P. D. Spanos 1991 Stochastic Finite Elements: A Spectral Approach. New York: Springer.
2. R. Moore 1966 Interval Analysis. Englewood Cliffs, NJ: Prentice-Hall.
3. G. Alefeld and J. Herzberger 1983 Introduction to Interval Computations. 111 Fifth Avenue, New York 10003: Academic Press, Inc.
4. R. B. KEARFOTT 1996 Euromath Bulletin 2, 95-112. Interval computations: Introduction, uses, and resources.
5. R. Chen and A. C. Ward 1997 Journal of Mechanical Design 119, 65-72. Generalizing interval matrix operations for design.
6. A. D. Dimarogonas 1995 Journal of Sound and Vibration 183, 739-749. Interval analysis of vibrating systems.
7. H. U. Koyluoglu, A. S. Cakmak and S. R. K. Nielsen 1995 Journal of Engineering Mechanics 1149-1157. Interval algebra to deal with pattern loading and structural uncertainties.
8. E. R. HANSEN 1992 SI AM Journal on Numerical Analysis 29, 1493-1503. Bounding the solution of interval linear equations.
9. U. Kulisch and W. Miranker 1983 A New Approach to Scientific Computation, Chapter Solving Algebraic Problems with High Accuracy (S. M. Rump, editor), 51-120. New York: Academic Press.
10. S. Ning and R. B. Kearfott 1997 SIAM Journal on Numerical Analysis 34, 1289-1305. A comparison of some methods for solving linear interval equations.
11. Z. Qiu, S. Chen and I. Elishakoff 1995 Journal of Optimization Theory and Applications 86, 669-683. Natural frequencies of structures with uncertain but non random parameters.
12. Z. Qiu, S. Chen and I. Elishakoff 1996 Chaos, Solitons and Fractals 7, 425-434. Bounds of eigenvalues for structures with an interval description of uncertain but non random parameters.
13. Z. Qiu, I. Elishakoff and J.S. Jr 1996 Chaos, Solitons and Fractals 7, 1845-1857. The bound set of possible eigenvalues of structures with uncertain but non random parameters.
14. Z. Qiu and I. Elishakoff 1998 Computer Methods in Applied Mechanics and Engineering 152, 361-372. Antioptimization of structures with large uncertain-but-non-random parameters via interval.
15. I. Elishakoff 1999 How to Find the Range of Eigenvalues Due to Uncertain Elastic Modulus and Mass Density (I. Elishakoff, editor), Whys and hows in uncertainty modelling, 341-355. Springer: Vienna
16. J. RoHn 1996 Technical Report 686. Institute of Computer Science, Academy of Sciences of the Czech Republic. Checking properties of interval matrices.
17. S.P. Shary 1995 SIAM Journal on Numerical Analysis 32, 610-630. On optimal solution of interval linear equations.
18. W. Oettli and W. Prager 1964 Numerische Mathematik 6, 405-409. Compatibility of approximate solution of linear equations with given error bounds for coefficients and right-hand sides.
19. J. Rohn 1995 Technical Report 619. Institute of Computer Science, Academy of Sciences of the Czech Republic. Np-hardness results for some linear and quadratic problems.
20. J. Rohn 1995 Technical Report 620. Institute of Computer Science, Academy of Sciences of the Czech Republic. Validated solutions of linear equations.
21. J. Rohn and G. ReX 1996 Technical Report 666. Institute of Computer Science, Academy of Sciences of the Czech Republic. Enclosing solutions of linear equations.
22. R. S. VARGA 1962 Matrix Iterative Analysis. Series in Automatic Computation. Englewood Chiffs, NJ: Prentice-Hall.
23. J.-F. Imbert 1995 Analyse des structures par éléments finis. Cépaduès Éditions, Toulouse, France.
24. S.-T. Wei and C. Pierre 1988 Journal of Vibration, Acoustics, Stress, and Reliability in Design 110, 429-438. Localisation Phenomena in mistuned assemblies with cyclic symmetry, Part 1: free vibrations.
25. S.-T. Wei and C. Pierre 1988 Journal of Vibration, Acoustics, Stress, and Reliability in Design 110, 439-449. Localisation phenomena in mistuned assemblies with cyclic symmetry, Part 2: forced vibrations.
26. O. KNÜppel 1994 Computing 53, 277-287. Profil/bias-a fast interval library.
27. O. KNÜPpel 1993 Bericht 93.3, Technische Universitat Hamburg-Harburg. Bias—basic interval arithmetic subroutines.

## APPENDIX A: OPERATIONS ON INTERVALS

As the interval arithmetic is different from the classical arithmetic, several arithmetic operations on intervals are defined.

The four classical arithmetic operations are also defined:

$$
\begin{align*}
\mathbf{x}+\mathbf{y} & =[\underline{\mathbf{x}}+\underline{\mathbf{y}}, \overline{\mathbf{x}}+\overline{\mathbf{y}}],  \tag{A1}\\
\mathbf{x}-\mathbf{y} & =[\underline{\mathbf{x}}-\overline{\mathbf{y}}, \overline{\mathbf{x}}-\underline{\mathbf{y}}],  \tag{A2}\\
\mathbf{x} * \mathbf{y} & =\left[\begin{array}{lll}
\min (\underline{\mathbf{x}} \underline{\mathbf{y}}, \underline{\bar{y}}, \overline{\mathbf{x}} \underline{y}, \overline{\mathbf{x y}}), \max (\underline{\mathbf{x}} \underline{y}, \underline{\mathbf{x}} \overline{\mathbf{y}}, \overline{\mathbf{x}} \underline{\mathbf{y}}, \overline{\mathbf{x y}})
\end{array}\right],  \tag{A3}\\
1 / \mathbf{x} & =\left[\begin{array}{lll}
1 / \overline{\mathbf{x}}, 1 / \mathbf{x}] & (0 \notin \mathbf{x}), \\
\mathbf{x} / \mathbf{y} & =\mathbf{x} *(1 / \mathbf{y}) \quad(0 \notin \mathbf{y}) .
\end{array}\right. \tag{A4}
\end{align*}
$$

An interval vector $\{\mathbf{x}\}$ is a vector whose components are intervals:

$$
\{\mathbf{x}\}=\left\{\begin{array}{c}
\mathbf{x}_{1}  \tag{A6}\\
\vdots \\
\mathbf{x}_{n}
\end{array}\right\} .
$$

An interval matrix [A] is a matrix whose components are intervals:

$$
\begin{equation*}
[\mathbf{A}]=\left[\mathbf{A}_{i j}\right], \quad i=1, \ldots, m, \quad j=1, \ldots, n . \tag{A7}
\end{equation*}
$$

## APPENDIX B: ALGORITHM OF RUMP

The problem to be solved is

$$
\begin{equation*}
[A]\{x\}=\{b\} . \tag{B1}
\end{equation*}
$$

$[A]$ is a square matrix. For an arbitrary non-singular matrix $[R]$, and a vector $\left\{x_{0}\right\}$,

$$
\begin{equation*}
[A]\{x\}=\{b\} \tag{B2}
\end{equation*}
$$

is equivalent to

$$
\begin{equation*}
\left\{x^{*}\right\}=[G]\left\{x^{*}\right\}+\{g\} \tag{B3}
\end{equation*}
$$

with

$$
\begin{gather*}
{[G]=[I]-[R][A],}  \tag{B4}\\
\{g\}=[R]\left(\{b\}-[A]\left\{x_{0}\right\}\right),  \tag{B5}\\
\{x\}=\left\{x_{0}\right\}+\left\{x^{*}\right\} . \tag{B6}
\end{gather*}
$$

In practice, $[R] \approx\left[A^{-1}\right]$, and $\left\{x_{0}\right\}=[R]\{b\}$, so that $[G]$ and $\{g\}$ are of small norms, and $\left\{x^{*}\right\}$ is close to 0 .

Let the interval vector $\{\mathbf{X}\}$ satisfy

$$
\begin{equation*}
[G]\{\mathbf{X}\}+\{g\} \subset\left\{\mathbf{X}^{0}\right\} \tag{B7}
\end{equation*}
$$

where $[G]\{\mathbf{X}\}+\{g\}=\{[G]\{X\}+\{g\} ;\{X\} \in\{\mathbf{X}\}\}$, and $\left\{\mathbf{X}^{0}\right\}$ is the interior of $\{\mathbf{X}\}$. Then,

$$
\begin{equation*}
\left\{x^{*}\right\}=[G]\left\{x^{*}\right\}+\{g\} \tag{B8}
\end{equation*}
$$

has a unique solution $\left\{x^{*}\right\} \in[G]\{\mathbf{X}\}+\{g\}[9]$.
The proof is true in the abstract, but the algorithm is used on computers that do not always give true results (due to the mantissa truncation). The programs used to compute interval arithmetic have to take the problem into account (for example the package BIAS from Knüppel [26,27]). If $\odot$ and $\oplus$ denote the computed interval multiplication and sum (they overestimate the true intervals), and

$$
\begin{equation*}
[G] \odot\{\mathbf{X}\} \oplus\{g\} \subset\left\{\mathbf{X}^{\mathbf{0}}\right\} \tag{B9}
\end{equation*}
$$

is true in computed interval arithmetic, then one has also

$$
\begin{equation*}
[G]\{\mathbf{X}\}+\{g\} \subset\left\{\mathbf{X}^{\mathbf{0}}\right\} \tag{B10}
\end{equation*}
$$

since

$$
\begin{equation*}
[G]\{\mathbf{X}\}+\{g\} \subset[G] \odot\{\mathbf{X}\} \oplus\{g\} . \tag{B11}
\end{equation*}
$$

The algorithm can be summarized as follows.
First, an initialization stage:
$\boldsymbol{\varepsilon}=[0 \cdot 9,1 \cdot 1]$ is the so-called inflation parameter;
$[R]=\operatorname{inv}(\operatorname{mid}[\mathbf{A}])$ is an estimation of the inverse of $\operatorname{mid}[\mathbf{A}]$;
$\left\{\mathbf{x}_{0}\right\}=[R] *\{\mathbf{b}\}$ is an estimation of the solution;
$\{\mathbf{g}\}=[R] *\left(\{\mathbf{b}\}-[\mathbf{A}] *\left\{\mathbf{x}_{0}\right\}\right) ;$
$\{\mathbf{x}\}=\{\mathbf{g}\}$ initialization of the solution $\left\{\mathbf{x}^{*}\right\} ;$
$[\mathbf{G}]=I-[R] *[\mathbf{A}]$ is the iteration matrix in the equation

$$
\begin{equation*}
\left\{\mathbf{x}^{*}\right\}=[\mathbf{G}]\left\{\mathbf{x}^{*}\right\}+\{\mathbf{g}\} . \tag{B12}
\end{equation*}
$$

Second, iterative resolution:
$\{\mathbf{y}\}=\boldsymbol{\varepsilon} *\{\mathbf{x}\} ;$
$\{\mathbf{x}\}=\{\mathbf{g}\}+[\mathbf{G}] *\{\mathbf{y}\}$
until $\{\mathbf{x}\} \subset\left\{\mathbf{y}^{0}\right\}$ or too many iterations;
if the condition $\{\mathbf{x}\} \subset\left\{\mathbf{y}^{0}\right\}$ is satisfied, then $\{\mathbf{x}\}$ is a conservative solution of the equation $[\mathbf{A}]\{\mathbf{x}\}=\{\mathbf{b}\}$.

## APPENDIX C: MODULUS OF THE TRANSFER FUNCTION

The modulus of the dynamic compliance vector is normally calculated as

$$
\begin{equation*}
|H|=\sqrt{H_{r}^{2}+H_{i}^{2}} \tag{C1}
\end{equation*}
$$

To avoid the problem of overestimation due to the dependence of the real and imaginary parts of the dynamic compliance, a method to compute the bounds of its modulus is proposed.

For a system with one interval parameter, the compliance vector could be written in the formalized way, after $N$ iterations, according to the recurrent scheme proposed in

Section 4 as

$$
\left\{\begin{array}{l}
\mathbf{H}_{i}^{N}  \tag{C2}\\
\mathbf{H}_{r}^{N}
\end{array}\right\}=\left\{\begin{array}{l}
\mathbf{H}_{i}^{N-1} \\
\mathbf{H}_{r}^{N-1}
\end{array}\right\}+\varepsilon^{N}\left\{\begin{array}{l}
\mathbf{X}_{i}^{N} \\
\mathbf{X}_{r}^{N}
\end{array}\right\},
$$

where $H_{i}^{N}$ is the imaginary part of the vector $H$, computed at the loop $N$ of the algorithm. The real and imaginary parts of $H$ are both depending on the same interval parameter $\varepsilon^{N}$, and applying directly equation ( C 1 ) would lead to large overestimations of $|H|$. Equation (61) can be written as

$$
\left\{\begin{array}{l}
\mathbf{H}_{i}^{N}  \tag{C3}\\
\mathbf{H}_{r}^{N}
\end{array}\right\}=\sum_{n=1}^{N} \boldsymbol{\varepsilon}^{n}\left\{\begin{array}{l}
\mathbf{X}_{i}^{n} \\
\mathbf{X}_{r}^{n}
\end{array}\right\}
$$

The modulus can then be calculated as

$$
\begin{equation*}
\bmod (\{\mathbf{H}\})=\sqrt{\sum_{n=1}^{N} \boldsymbol{\varepsilon}_{1}^{2 n}\left(\mathbf{X}_{i}^{n 2}+\mathbf{X}_{r}^{n 2}\right)+2 \sum_{n=1}^{N-1} \sum_{p=n+1}^{N} \boldsymbol{\varepsilon}_{1}^{n+p}\left(\mathbf{X}_{i}^{n} \mathbf{X}_{i}^{p}+\mathbf{X}_{r}^{n} \mathbf{X}_{r}^{p}\right)}, \tag{C4}
\end{equation*}
$$

and the dependence between the real and imaginary parts is preserved in a better way than applying equation (C1) directly.

## APPENDIX D: NOMENCLATURE

| $x$ | scalar |
| :--- | :--- |
| $\{x\}$ | vector |
| $[A]$ | matrix |
| $\mathbf{x}$ | interval |
| $\overline{\mathbf{x}}$ | interval lower bound |
| $\overline{\overline{\mathbf{X}}}$ | interval upper bound |
| $\operatorname{rad}(\mathbf{x})$ | interval radius |
| $m(\mathbf{x})=\mathbf{x}_{\mathrm{c}}=\frac{\mathbf{x}+\overline{\mathbf{x}}}{2}$ | interval center |
| $w(\mathbf{x})=\overline{\mathbf{x}}-\underline{\mathbf{x}}$ | interval width |
| $\{\mathbf{x}\}$ | interval vector |
| $[\mathbf{A}]$ | interval matrix |
| $[M]$ | mass matrix |
| $[K]$ | stiffness matrix |
| $\eta$ | hysteretic damping coefficient |
| $E$ | Young's modulus |
| d.o.f. | degrees of freedom |
| flops | floating point operations |

